EXPERIMENTAL STUDY ON THE CYCLIC BEHAVIOUR OF BOLTED END-PLATE JOINTS

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ABSTRACT

In this paper an experimental study is reported performed on end-plate type joints. The test arrangement represents a column-base joint of a steel frame. Altogether six specimens were tested, each of them subject to cyclic loading. The specimens were carefully designed, by performing detailed preliminary calculations, so that they would present the typical behaviour types of end-plate joints. On the basis of the experimentally established moment-rotation relationship the cyclic characteristics of each specimen have been calculated and compared to one another. The results are evaluated, qualitative and quantitative conclusions are drawn.

1 INTRODUCTION

End-plate-type joints are widely used in steel frame structures. These joints can connect either two steel elements (like beam-to-column, beam-to-beam or column-to-column joints) or a steel and a concrete/reinforced concrete element (like column-base joints or joints of a steel beam and a reinforced concrete column). The main advantage of this type of joint is in the production and mounting, at the same time, however, their application results in a more complicated structural behaviour and, consequently, requires more complex design.

In the recent decade lots of investigation (both numerical and experimental) have been performed to analyse the behaviour of the different kinds of end-plate joints, and to develop appropriate numerical models for the everyday engineering practice. As a result of these investigations, focused mainly on the monotonic behaviour, calculation methods have been developed and introduced to the new European steel code for steel-to-steel joints (Eurocode 3, 1991), as well as design tables are worked out for column-bases (Wald et al, 1994). To the cyclic behaviour, however, much less efforts have been devoted. Nevertheless, certain number of experimental programs have been performed (Dunai et al. 1996, Calado et al. 1999, Ballio et al. 1997, Calado and Lamas 1998, Calado et al. 1998), as well as some numerical models have been developed (Ádány and Dunai 1995, Ádány and Dunai 1997, Dunai 1992, Dunai et al. 1995, Dunai and Ádány 1997). The lack of appropriate numerical models can be explained by the rather complicated joint behaviour which requires sophisticated models and time-consuming calculations.

The more complete understanding of the cyclic behaviour of end-plate joints is essential, especially in the seismic design. The importance of the problem was clearly justified during the recent earthquake events of Northridge and Kobe, where significant structural damage of steel frames took place in the connection zones in several cases. Thus, it is important to be able to simply but reliably asses the behaviour of the joints in case of seismic actions, in order to satisfy the required resistance, rigidity, ductility and energy absorption demands.

In this paper an experimental program is reported, carried out in the Instituto Superior Técnico, Lisbon, Portugal. The test program is devoted to the cyclic behaviour of column-base type end-plate joints. The paper presents the whole process of the experimental program. In Section 2 the aim of the program is defined, in Section 3 the preliminary work is summarised, in Section 4 the results are presented. Finally some conclusions are drawn.

2 AIM

According to the previous experiments four basic behaviour components can be distinguished which determine the joint behaviour, usually measured by the moment-rotation relationship. These components, illustrated in Figure 1, are the followings:

- end-plate behaviour, due to the elastic/plastic deformation of end-plate,
- bolt behaviour, due to the elastic/plastic elongation of bolts,
- steel beam/column behaviour, due to the elastic/plastic deformations of the connected steel element, including local buckling,
- concrete behaviour, characterised by the deterioration of the concrete element.

It is to be noted that other reasons of failure can also occur, like weld failure, it is reasonable to assume, however, that these phenomena have no significant effect on the behaviour before the failure, as well as they can be eliminated by appropriate design and fabrication.

The primary aim of the experimental program is to provide information on the joint behaviour, including

- joint behaviour governed by the *end-plate behaviour,*
- joint behaviour governed by the *bolt behaviour,*
- joint behaviour governed by the *beam/column behaviour,*
- joint behaviour governed by the *interaction of them.*

It is important to underline that the concrete behaviour is out of the scope of this study.

The experimental data, provided by the tests, can also be used to verify and calibrate numerical models, which is another important aim of the experimental program.

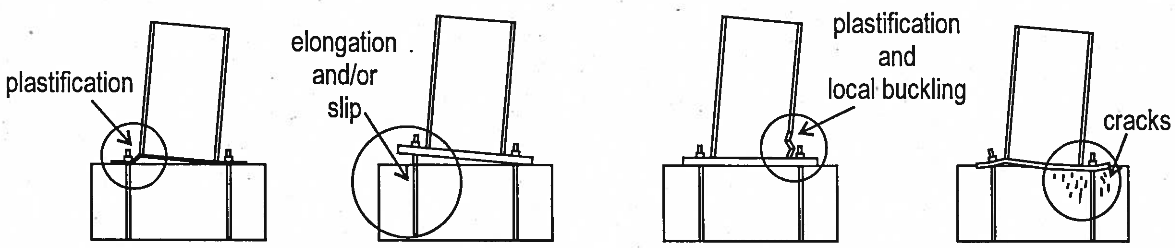

Figure 1 Behaviour components of the joint behaviour

3 DESIGN OF THE TESTS

3.1 Test equipment

The test equipment is basically developed to test beam-to-column joints of steel frames. The global arrangement is illustrated in Figure 2. In addition, there is a lateral frame to make possible the lateral support of the specimen, avoiding its lateral movement or twisting. The whole testing process is managed by a personal computer, by means of a data acquisition unit, which commands the actuator and reads the data from the load cell and displacement transducers. Another important note that displacement control is used. (More information about the test equipment can be found in Ferreira 1994 and Calado and Mele 1999.)

In designing the test, the specimen characteristics are determined in accordance with the parameters of the existing test set-up, by considering the geometrical properties, the load capacity of the actuator and load cell, as well as the displacement capacity of the inductive displacement transducers. The global arrangement is presented in Figures 2 and 3, with the main geometrical dimensions of the specimens in Figure 2. The arrangement represents a column base joint, with an H-shaped column and a practically rigid base. The application of the base element is necessary to be able to connect the column to the base beam. The top part of the specimen has the role to ensure the restraint against lateral movement and twisting of the column.

3.2 Preliminary calculations

To be able to achieve the intended phenomena of the specimens preliminary calculations were done. The moment resistance of each joint was determined on two-dimensional models as the minimum of resistances belonging to the possible failure modes.

Four modes of failure were defined as illustrated in Figure 5. Mode 1 represents

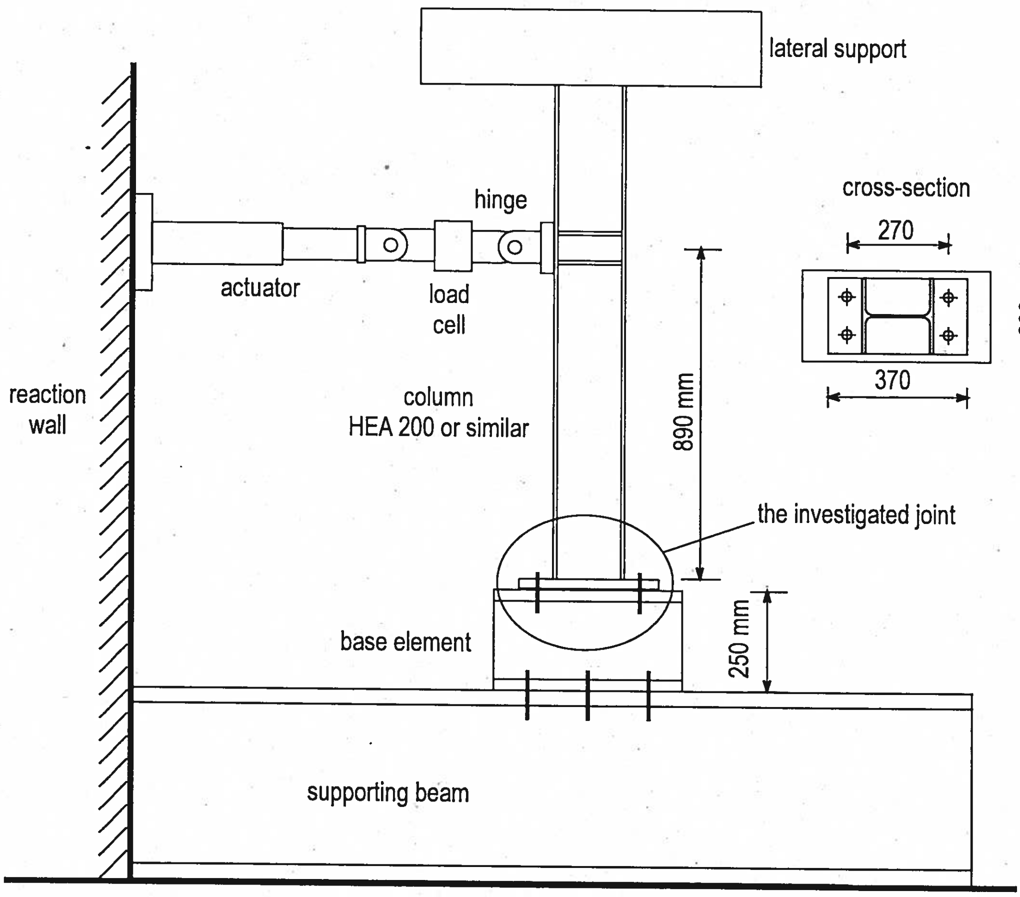

Figure 2 Global arrangement with the main dimensions of the specimens

Figure 3 Global arrangement of the test equipment

the pure end-plate failure without failure of bolts. Mode 4 corresponds to the pure bolt failure, without any failure of the end-plate. Mode 2 and 3 are two cases of combined bolt and end-plate failure.

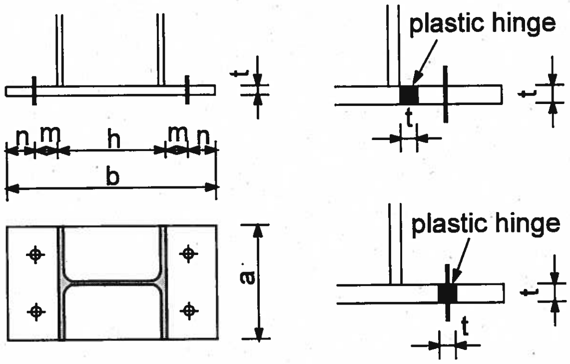

Figure 4 Notations and assumed position of plastic hinges

There are two potential places where plastic hinge (hinge-line) can be developed: either at the bolts, or at the column flange. The assumed positions of the plastic hinges are presented in Figure 4.

The joint resistance belonging to the various failure modes can be expressed by the following formulae, for Modes 1 to 4, respectively.

$$M_{Rd,1} = M_{ep,Rd} \cdot 2 \cdot \left(1 + \frac{h'}{m'}\right)$$

$$M_{Rd,2} = M_{ep,Rd} \cdot \left(2 + \frac{h'}{m'+n}\right) + \\ + F_{b,Rd} \cdot \frac{h' \cdot n}{m'+n}$$

$$M_{Rd,3} = M_{ep,Rd} + F_{b,Rd} \cdot (h' + m')$$

$$M_{Rd,4} = F_{b,Rd} \cdot b$$

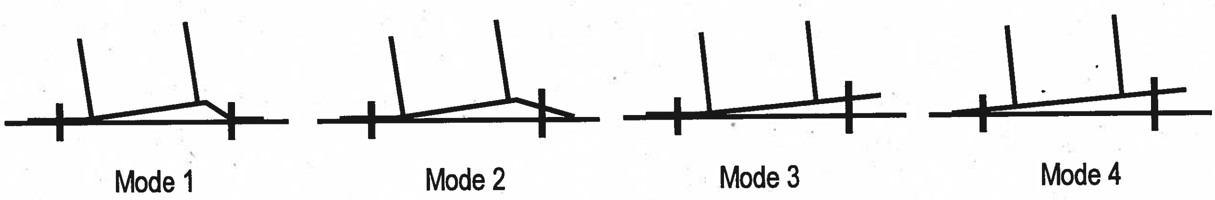

Figure 5 Possible failure modes of end-plate joints

Table 1 Moment resistance calculation of the joints

t [mm]	Plate res. [kNm]	Mode 1 [kNm]	Mode 2 [kNm]	Mode 3 [kNm]	Mode 4 [kNm]	Resistance [kNm]
10	1.35	18.1	33.0	55.5	85.2	18.1
12	1.944	27.0	36.3	56.3	85.2	27.0
16	3.456	51.4	44.5	58.3	85.2	44.5
20	5.4	86.4	55.2	60.7	85.2	55.2
22	6.534	108.6	61.5	62.1	85.2	61.5
25	8.4375	148.8	72.2	64.3	85.2	64.3
30	12.15	238.1	93.7	68.6	85.2	68.8
40	21.6	540.0	152.0	79.2	85.2	79.2
50	33.75	1147.5	234.7	92.5	85.2	85.2

The notations are as follow:

- $M_{ep,Rd}$ denotes the plastic resistance of the end-plate, calculated as:

$$M_{ep,Rd} = \frac{a \cdot t^2}{4} \cdot f_y$$

- $F_{b,Rd}$ denotes the plastic resistance of the bolts (two bolts), calculated as:

$$F_{b,Rd} = 2 \cdot A_b \cdot f_{yb}$$

- h' and m' can be calculated as:

$$h' = h + t$$

$$m' = m - \frac{t}{2}$$

- a, b, h, m, n, and t are geometrical dimensions of the joint, presented in Figure 4,
- A_b is the sectional area of one bolt,
- f_y and f_{yb} the yield stress of the base material and bolt material, respectively.

The applied bolt is of grade 8.8, while the material is S235, which means that the characteristic value of the yield strength is equal to 640 MPa and 235 MPa for the bolt and the base material, respectively, according to the Eurocode 3 (1991). Generally, these characteristic values are adopted as the basis of further calculations. According to previous experiences, however, a higher value is considered for the base material (270 MPa).

The column section is assumed to be a HEA 200 profile, or similar, the height of which is h = 190 mm. For the bolt position and extension of end-plate m = 40 mm and n = 50 mm are applied. The thickness of the end-plate is treated as a parameter, varying between 10 and 50 mm. The bolt diameter is 16 mm, which gives approximately A_b = 200 mm² for the area of one bolt.

The calculations are summarised in Table 1, showing the resistances of the various failure modes for the various plate thickness values. The most probable failure mode is the one to which the minimal resistance belongs.

It can be seen from Table 1 that pure bolt failure is not realistic to achieve since it occurs only in case of extremely thick end-plate. For that reason it was decided to eliminate the pure bolt failure from the study and, finally, three pieces of end-plate thickness were chosen, according to failure mode 1, 2 and 3. These are:

- t = 12 mm, for Mode 1,
- t = 16 mm, for Mode 2,
- t = 25 mm, for Mode 3.

In addition, it was decided to study the effect of bolt pre-tensioning. It can be assumed, however, that the resistance is not dependent on the pre-loading.

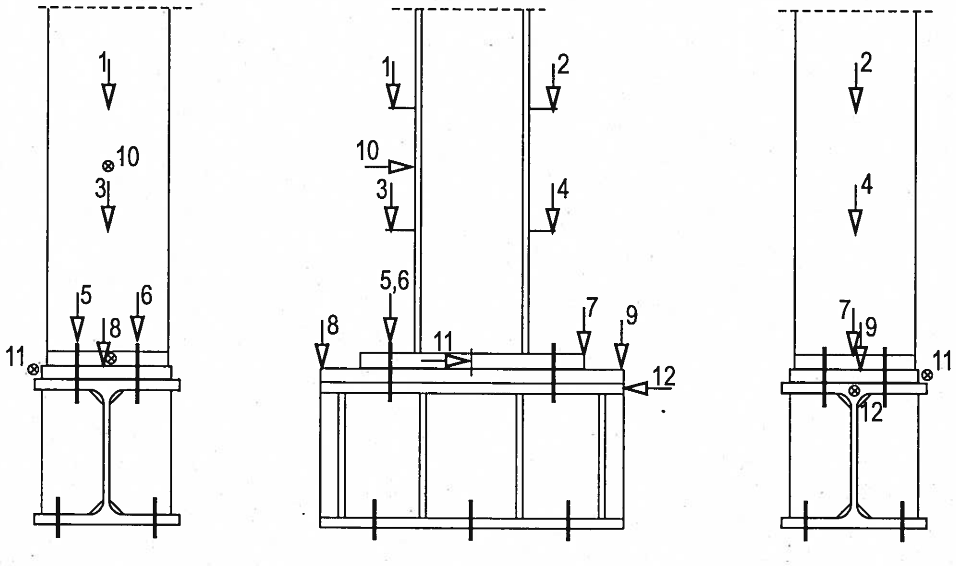

Figure 6 Arrangement of displacement transducers

3.3 The specimens

Altogether five specimens were designed. The main characteristics of the specimens are summarised in Table 2. An additional note that butt welds are applied between the column and the end-plate in order to minimise the risk of weld failure.

3.4 Displacement transducers

To measure the displacements inductive displacement transducers were used. The number and the position of the transducers were defined so as to get the possible most information on the behaviour, considering also the place required for each transducer, which gives a limitation of their maximal number. Finally, altogether 13 transducers were used. Their arrangement from T1 to T12 is presented in Figure 6.

Moreover, there is a transducer, not presented in Figure 6, to measure the horizontal displacement of load application point. Since all the test procedure is controlled by this displacement, it is referred as transducer T0.

3.5 Loading history

The loading history is defined in accordance with (ECCS 1986) on the basis of the displacement belonging to the limit of elasticity (e_y). In this study, however, two cycles were applied in the plastic range instead of three as proposed in the (ECCS 1986), because it was observed in previous tests (Calado and Lamas 1998) that the third cycle does not give additional information relatively to the previous two cycles. The general pattern of the loading is summarised in Table 3.

Table 2 Specimens main characteristics

Specimen	Column section	End-plate thickness	Bolt tightening	Anticipated behaviour
CB1 / CB1R	HEA200	25 mm	hand-tightened	Mode 3
CB2	HEA200	16 mm	hand-tightened	Mode 2
CB3	welded	25 mm	hand-tightened	local buckling
CB4	HEA200	25 mm	pre-tensioned	Mode 3
CB5	HEA200	12 mm	hand-tightened	Mode 1

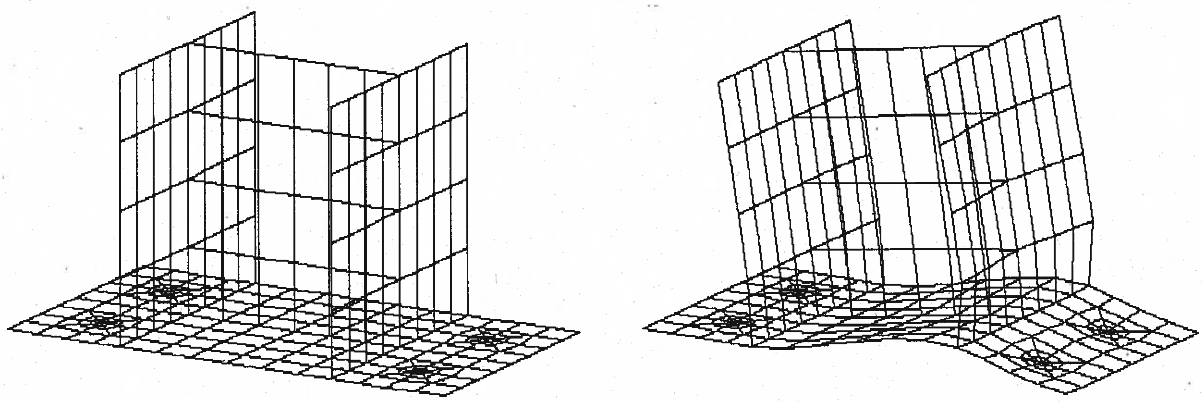

Figure 7 Undeformed and deformed grid of the FE model for CB2

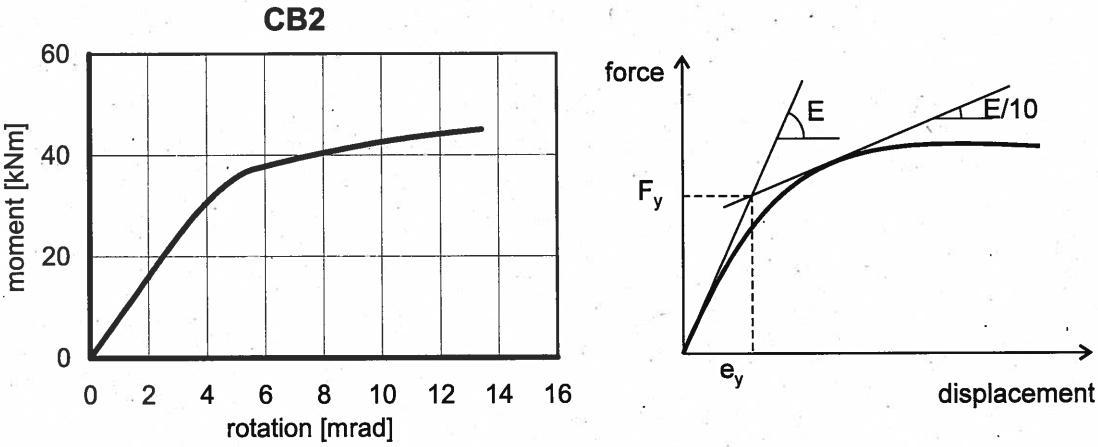

Figure 8 Calculated monotonic moment-rotation diagram for CB2, and the determination of limit of elasticity

3.6 Determination of limit of elasticity

Before performing the cyclic test, the yielding displacement (e_y) have to be determined. It can be done by monotonic test, as it is proposed in ECCS 1986, or by preliminary calculations. In this experimental program a finite element simulation was performed, using a model developed by the authors. (see Dunai and Ádány 1997) As an example, Figure 7 shows the FE model of CB2, with the applied finite element mesh. Only the bottom part of the specimen was modelled (a 200-mm-high piece of the column), applying prescribed forces on the top of the model.

The calculated moment-rotation curve is presented in Figure 8. On the basis of the moment-rotation relationship, the force-displacement relationship can be established. From the force-displacement curve the yielding displacement and yielding force was determined according to the ECCS recommendations as it is illustrated in Figure 8.

Table 3 Loading history

Cycle nr.	1	2	3	4	5	6	7	8	9	10	11	
Displacement amplitude	$\frac{1}{4}e_y$	$\frac{1}{2}e_y$	$\frac{3}{4}e_y$	e_y	$2e_y$	$2e_y$	$3e_y$	$3e_y$	$4e_y$	$4e_y$	$5e_y$	etc.

4 RESULTS

4.1 Summary on the tests

At test CB1 (HEA 200 column, 25-mm-thick end-plate, hand-tightened bolts, see Table 2) the governing phenomenon of the behaviour is the elastic-plastic elongation of the bolts. The plastification, however, takes place not in the bolt shank but in the bolt and nut threads. Due to the deterioration of the nut thread, at the end of cycle 7 the nuts of the tensioned bolts were "jumped" from the bolt, which means joint failure.

Since the specimen has not significantly deteriorated during the test it was decided to replace the bolts, applying more nuts (3 pcs), and repeating the test. The repeated test is referred as CB1R.

In case of CB1R (see Table 2 and Figure 9) the behaviour is basically identical with that of CB1, following Mode 3 (see Section 3.2). The most important phenomenon is the bolt elongation, combined with certain end-plate deformation. In this case, however, the bolt plastification takes place in the bolt shank, which results in having more cycles performed. At the end of cycle 13 one of the tensioned bolts broke which means the failure of the joint.

In case of CB2 (HEA 200 column, 16-mm-thick end-plate, hand-tightened bolts, see Table 2 and Figure 10) specimen the behaviour follows Mode 2 (see Section 3.2). There is a strong interaction between two basic phenomena: the base-plate deformation and the bolt elongation. Both components reach its plastic state, due to plate bending for the base-plate, and a combined tension/bending for the bolt. At the very end of cycle 16 the flange weld was broken, causing the failure of the joint.

At CB3 test (welded column, 25-mm-thick end-plate, hand-tightened bolts, see Table 2) the governing phenomenon is the buckling of the column flanges and web, as it can be seen in Figure 11. There is no considerable deformation of the bolt, and it remains practically elastic during the whole test. Similarly, the base-plate remains

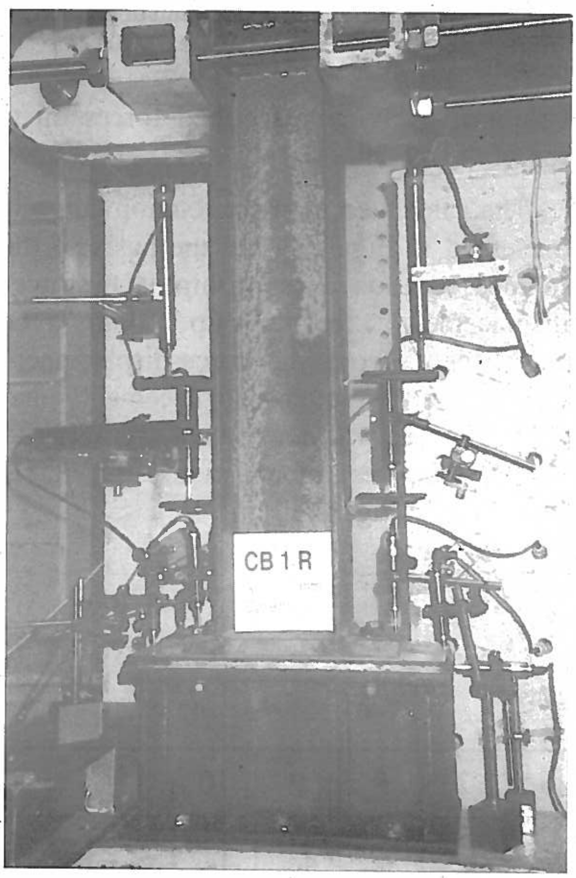

Figure 9 CB1R test

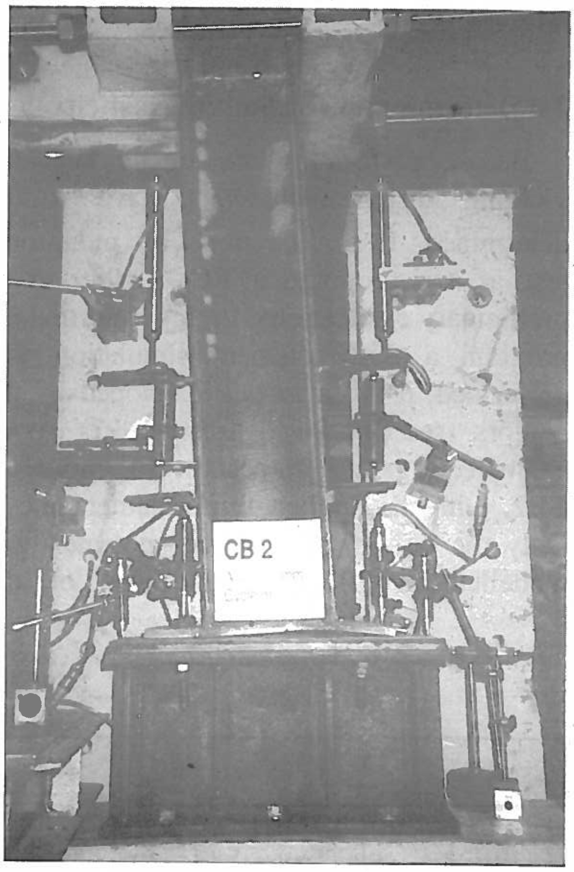

Figure 10 CB2 test

practically flat, without plastic deformations. The large deformations are concentrated in the bottom part of the column, approx. 300 mm from the end-plate. It is to be noted that the deformation pattern of the column is not exactly symmetrical. It is interesting to observe that there is a considerable shortening of the column due to the flange/web buckling. (The maximum shortening is approx. 25 mm.) After the 27th cycle the test was decided to stop, since the majority of transducers reached their measuring capacity, due to the large deflections.

CB4 specimen (HEA 200 column, 25-mm-thick end-plate, pre-tensioned bolts, see Table 2) is identical with CB1 and CB1R, with the only difference of bolt pre-tensioning. The behaviour is similar to that of CB1R, governed by the bolt elongation. At cycle 11 one of the tensioned bolts is broken, as it is presented in Figure 12. The test was not continued, because of the significant degradation of the joint load-bearing capacity.

For CB5 (HEA 200 column, 12-mm-thick end-plate, hand-tightened bolts, see Table 2) the governing phenomenon is definitely the base-plate deformation (elastic and plastic). Nevertheless, the bolt elongation has also an influence, especially after some plastic cycles. The behaviour is between Mode 1 and Mode 2 (see Section 3.2). Nevertheless, the prying effect was clearly observed, especially at larger displacements.

In cycle 11 a small crack occurred in the end-plate beside the flange weld, along the whole width of the end-plate. At the end of cycle 14 a similar crack developed at the opposite side of the column. The development of the weld cracks resulted in a degradation of the resistance. At cycle 18 the joint failed, caused by the complete failure of end-plate (see Figure 13).

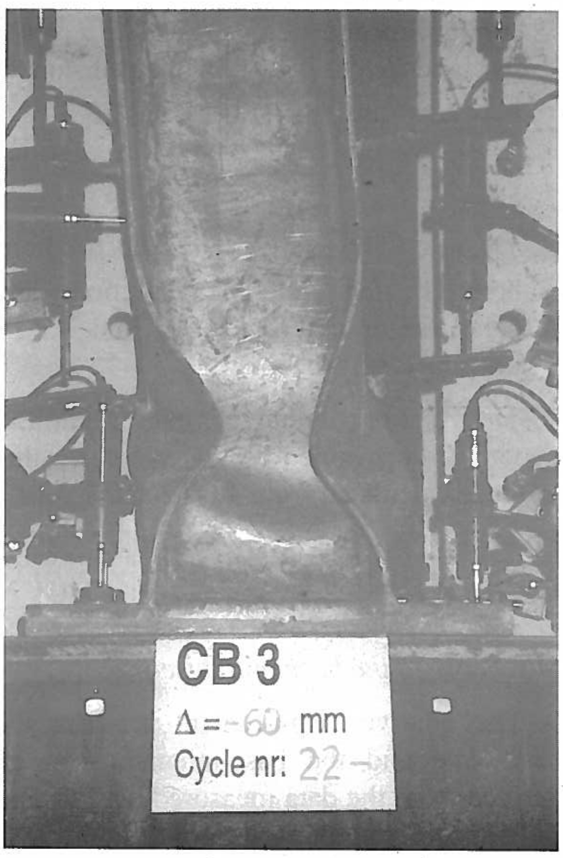

Figure 11 CB3 test

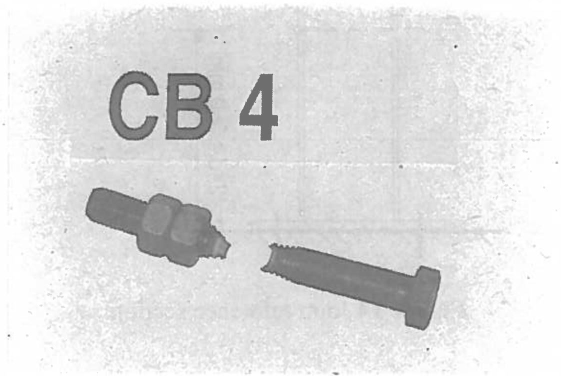

Figure 12 Failed bolt from CB4 test

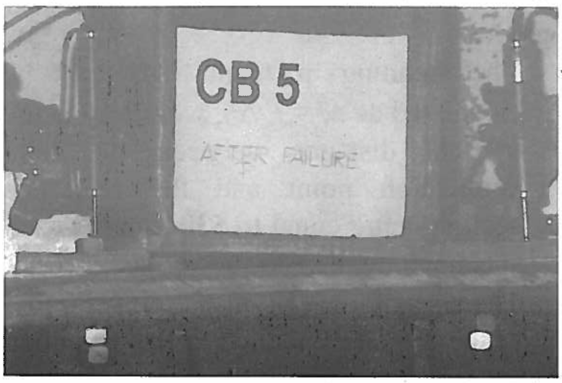

Figure 13 End-plate failure in CB5 test

4.2 Procedure for results evaluation

Definition of the joint reference section

In order to be able to calculate the various mechanical characteristics of the investigated column-base joint, it is necessary to define a section as *joint reference section*. This section is used to measure the forces/moments which act on the joint, and also the displacements of this section give the joint displacements.

The joint reference section is defined at a distance of twice the column section depth ($h_{ref} = 2 \times d$) in order to be not disturbed by the intensive deformation due to local buckling. Note that h_{ref} is measured from the top surface of the base-plate, while d is the distance between the system lines of the flanges, as it is illustrated in Figure 14. Since d is equal to 180 mm for all the specimens, the reference section is situated 360 mm from the base-plate.

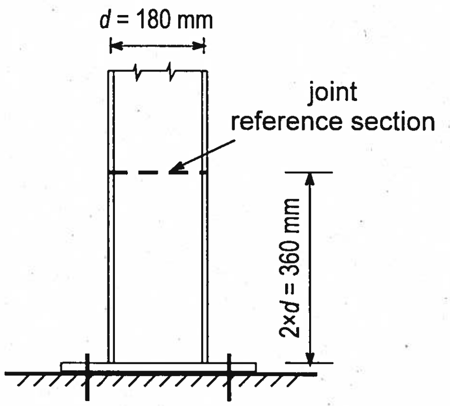

Figure 14 Joint reference section

Forces, moments

The joint forces/moments are defined as the internal forces/moments of the reference section.

- The bending moment (M) can be calculated as $M = F \cdot \Delta h$, where Δh is the vertical distance between the load application point and the reference section being equal to 530 mm for each specimen, while F is the applied force.
- The shear force (V) of the joint is equal to the force (F) applied to the specimen at the column top.

Base rotation

The rotation of the base can be calculated from the vertical displacements of the base element as:

$$\Theta_{base} = \left(t_9 - t_8\right) / d_{8-9} ,$$

where t_8 and t_9 are the displacements measured by T8 and T9, and $d_{8\text{-}9}$ is the distance between the two transducers.

Rotation of the reference section

The rotation of the reference section can be calculated from the measured vertical displacements of the flanges, which are recorded at two different levels. The elastic column deformations should also be considered. The rotation calculated from T1-T2 and T3-T4:

$$\Theta_{1-2} = \frac{t_2 - t_1}{d_{1-2}} - \Theta^{el}_{1-2} - \Theta_{base}$$

$$\Theta_{3-4} = \frac{t_4 - t_3}{d_{3-4}} + \Theta^{el}_{3-4} - \Theta_{base}$$

where

t_i is the displacement measured by the i-th transducer,

$d_{1\text{-}2}$ and $d_{3\text{-}4}$ are the horizontal distances between transducer T1-T2 and T3-T4,

Θ_{base} is the base rotation as defined above,

Θ^{el}_{1-2} and Θ^{el}_{3-4} are the elastic rotations between the reference section and measuring point for T1-T2 and T3-T4 transducers, respectively.

The rotation of the reference section is generally determined as the average value of those calculated from T1-T2 and T3-T4.

$$\Theta_{ref} = \frac{\Theta_{1-2} + \Theta_{3-4}}{2}$$

Slip between the base and base-plate

The relative displacement between the base-plate and the base element can be derived from the data measured by T11 and T12, considering the appropriate signs.

$$sl = t_{11} - t_{12}$$

Bolt elongation

The bolt elongation can be determined directly from the displacement measured by T5 and T6. However, it is enough to consider the positive displacements only.

$$el = \frac{t_5 + t_6}{2}$$

4.3 Moment-rotation characteristics

The moment-rotation relationship of the reference section is established on the basis of the measured displacements, according to the formulae presented in Section 4.2. The moment-rotation curves are presented in Figure 15.

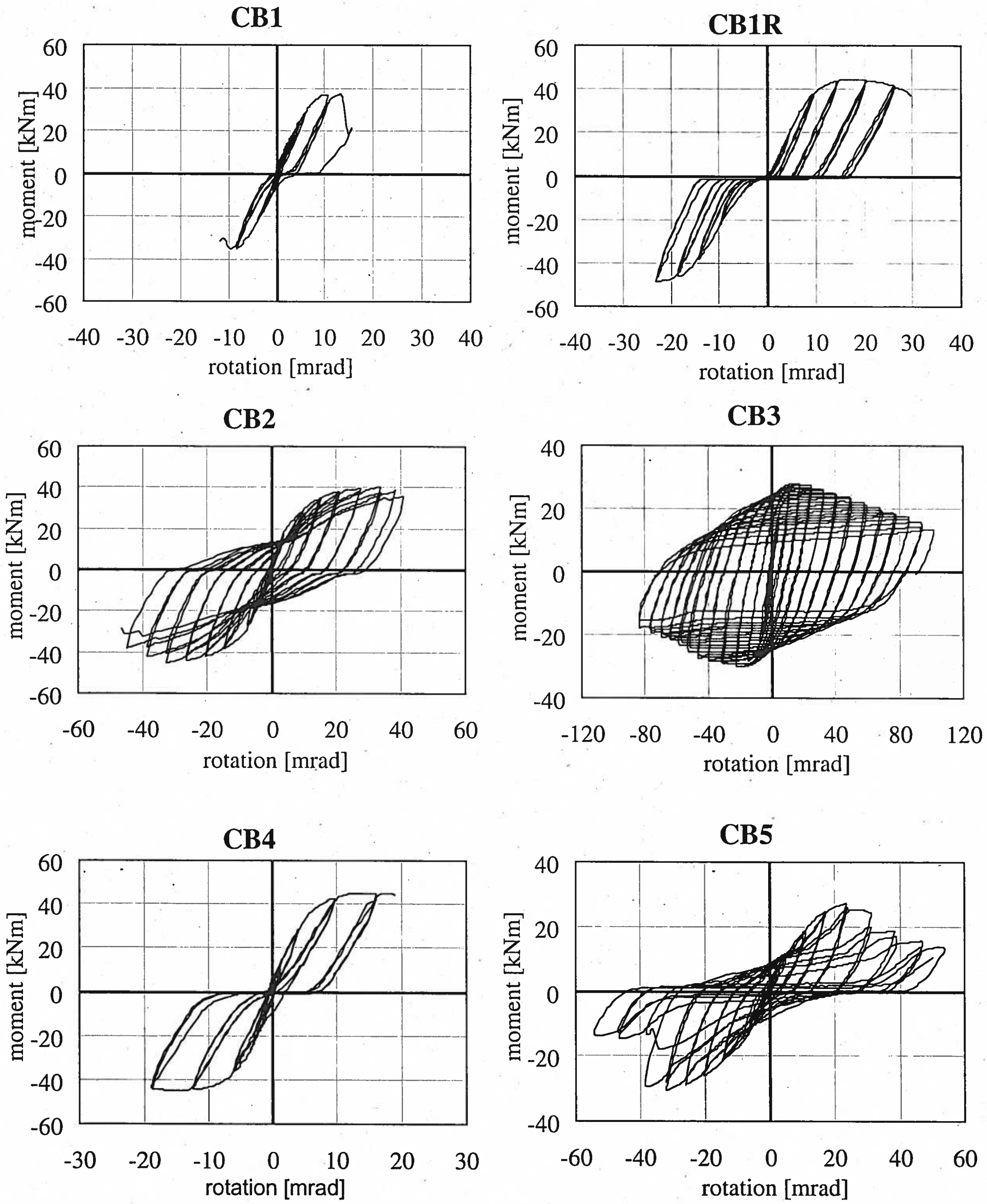

Figure 15 Moment-rotation diagrams

4.4 Base deformation

A general observation is that the deformations of the base element are not significant. They always remain in the elastic domain, with a maximum rotation of approx. 2-2.5 mrad, and a maximum horizontal displacement of approx. 1 mm. Note, that the yielding rotation is between 4 and 12 mrad, which means that the base deformation has no significant effect in the plastic range, but cannot be neglected in the vicinity of elastic range.

As an example, Figure 16 shows the rotation of the base element for CB2 and CB3. Similar diagrams are obtained for all the specimens, which clearly demonstrates that the base deformations are elastic, without having considerable effect on cyclic joint behaviour.

4.5 Slip between the base and base-plate

Figure 17 shows the slip-force diagram for CB2 and CB3 specimens. (Here, the term "force" is the shear force.) It can be observed that the development of the slip begins at the zero-force level of each cycle, and very rapidly reaches its maximum value. This observation can be easily explained by taking into consideration that, at the zero-force level, the contact area between the base-plate and the base element is reduced due to the residual deformations of base-plate.

Similar diagrams can be obtained for all the cases where significant base-plate or bolt deformation is experienced (CB1, CB1R, CB2, CB4, and CB5). However, in case of CB3, the effect of slip is almost negligible, since the base-plate and the bolts are not subjected to plastic deformations.

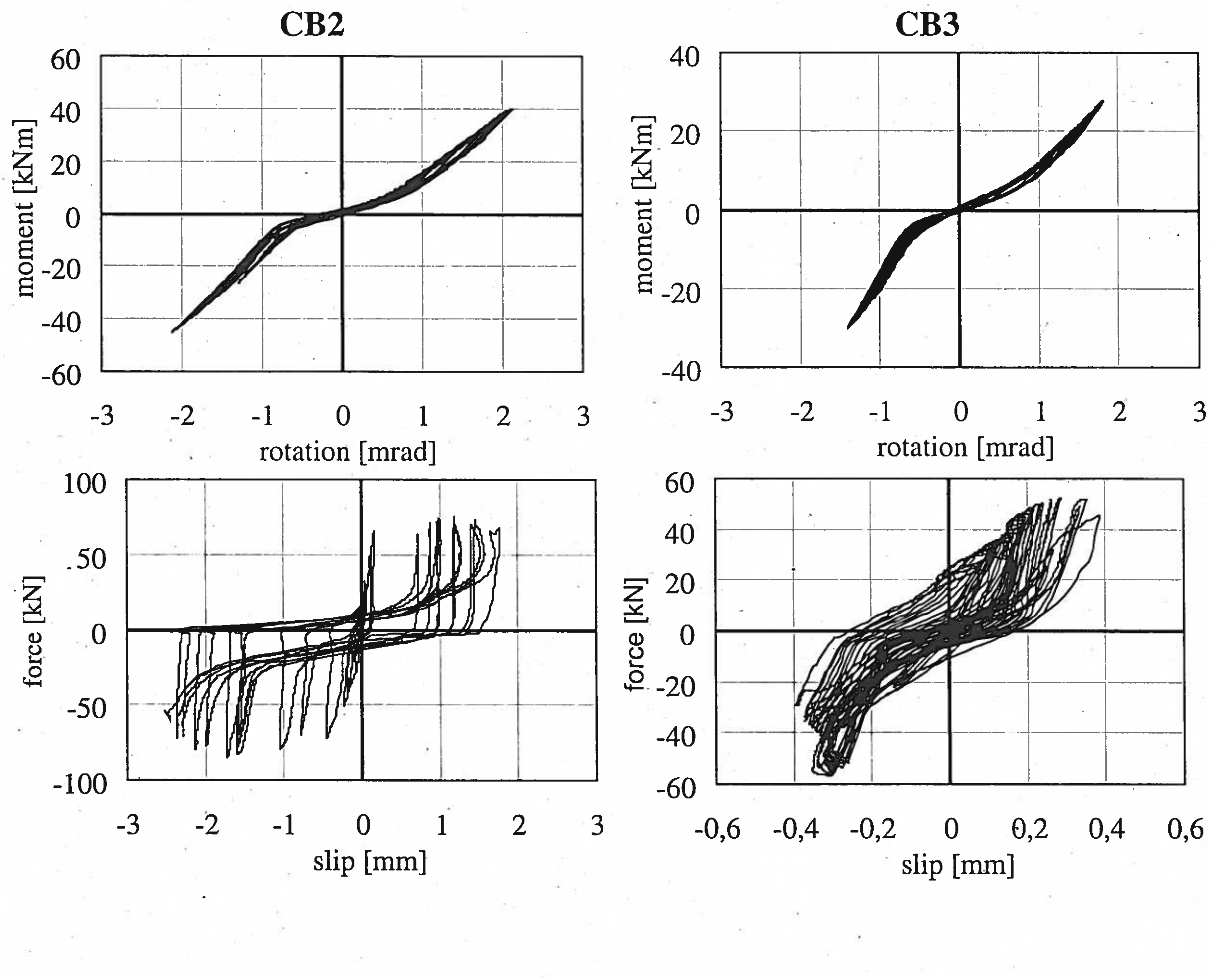

Figure 17 Slip between the base and base-plate for CB2 and CB3

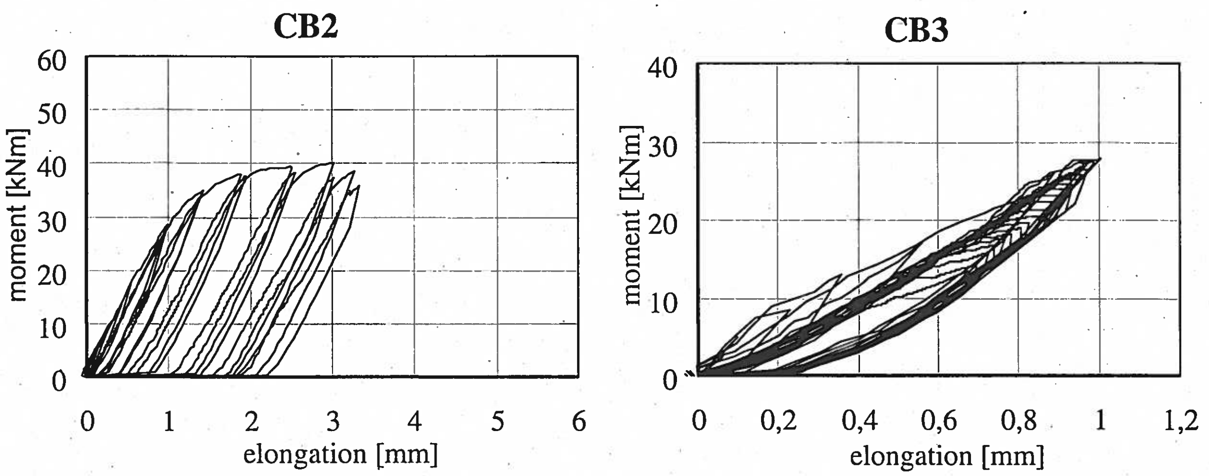

Figure 18 Bolt elongation for CB2 and CB3

4.6 Bolt elongation

Figure 18 presents the bolt elongation in case of CB2 and CB3 specimens. The diagram which was obtained for CB2 test well demonstrates the yielding of the bolts, as well as the significant residual deformations. Similar diagram can be shown for CB1R and CB4, too, where considerable bolt elongation occurred due to plastic deformations. In case of CB3 and CB5, however, the bolts remain practically elastic, consequently, there are no large deformations, nor residual displacements.

4.7 Cyclic characteristics

The moment-rotation curve, being the most important cyclic curve of the investigated type of joint, is chosen to establish the cyclic characteristics, as it is proposed in ECCS (1986). Four parameters are calculated: the *full ductility ratio*, the *resistance ratio*, the *rigidity ratio* and the *absorbed energy ratio*. In the followings, these ratios are plotted in function of the *partial ductility*. The formulae are presented in Table 4, according to the notations of Figure 19.

Table 4 Formulae for calculation of cyclic parameters

	Positive hemicycles	Negative hemicycles
Partial ductility	$\mu_{0i}^{+} = \dfrac{\Theta_i^{+}}{\Theta_y^{+}}$	$\mu_{0i}^{-} = \dfrac{\Theta_i^{-}}{\Theta_y^{-}}$
Full ductility ratios	$\psi_i^{+} = \dfrac{\Delta\Theta_i^{+}}{\Theta_i^{+} + \Theta_i^{-} - \Theta_y^{-}}$	$\psi_i^{-} = \dfrac{\Delta\Theta_i^{-}}{\Theta_i^{-} + \Theta_i^{+} - \Theta_y^{+}}$
Resistance ratios	$\varepsilon_i^{+} = \dfrac{M_i^{+}}{M_y^{+}}$	$\varepsilon_i^{-} = \dfrac{M_i^{-}}{M_y^{-}}$
Rigidity ratios	$\xi_i^{+} = \dfrac{\mathrm{tg}\,\alpha_i^{+}}{\mathrm{tg}\,\alpha_y^{+}}$	$\xi_i^{-} = \dfrac{\mathrm{tg}\,\alpha_i^{-}}{\mathrm{tg}\,\alpha_y^{-}}$
Absorbed energy ratios	$\eta_i^{+} = \dfrac{A_i^{+}}{M_y^{+} \cdot \left(\Theta_i^{+} - \Theta_y^{+} + \Theta_i^{-} - \Theta_y^{-}\right)}$	$\eta_i^{-} = \dfrac{A_i^{-}}{M_y^{-} \cdot \left(\Theta_i^{+} - \Theta_y^{+} + \Theta_i^{-} - \Theta_y^{-}\right)}$

The calculated cyclic parameters are presented in Figure 20 for the positive hemicycles of the moment-rotation diagrams. Note that CB1 test is not evaluated here, since, due to the early failure of the bolt and nut thread, the loading history is too short.

Effect of bolt pre-tensioning

CB1R and CB4 specimens are identical. However, in case of CB1R the bolts are hand-tightened, while in case of CB4 pre-tensioning is applied. Thus, the effect of bolt pre-tensioning can be analysed by comparing the two cases.

The behaviours observed during the two tests are similar, as well as the calculated cyclic parameters have similar tendencies, according to Figure 20. Thus, it can be stated that the bolt pre-tensioning has no important effect on the cyclic behaviour in the analysed cases.

Deformation capacity

An important observation is that the deformation capacities of the various tested joints strongly differ from each other.

- If the governing behaviour is the bolt behaviour (Mode 3, see CB4 or CB1R), the maximal value of the partial ductility is approx. equal to 4. In this case the failure of the specimen is caused by the failure of the bolts. Since the bolts are of high-strength steel, the deformation capacity of the bolts is limited, which results in a small deformation capacity of the joint.
- In case of CB2 there is a strong interaction between the bolt and end-plate (Mode 2). The maximal value of the partial ductility is 6. In this case the failure is caused by the crack occurred at the flange to end-plate welds. It means, that although the behaviour itself is governed by the end-plate and bolt deformations, the failure is caused by the weld crack, which limits the deformation capacity of the joint.
- CB 5 corresponds to Mode 1, when the behaviour is basically governed by the end-plate deformations. The maximal partial ductility is slightly more than 4. It is to be noted, however, that the failure is caused by the failure of the flange weld again, which reduced the deformation capacity.
- If the governing behaviour is the column flange/web buckling (CB3), the maximal partial ductility is much more than any of the other cases (more than 20). Moreover, it should be mentioned that CB3 joint has not reached its deformation capacity during the test. (The test was finished because of the measuring devices.)

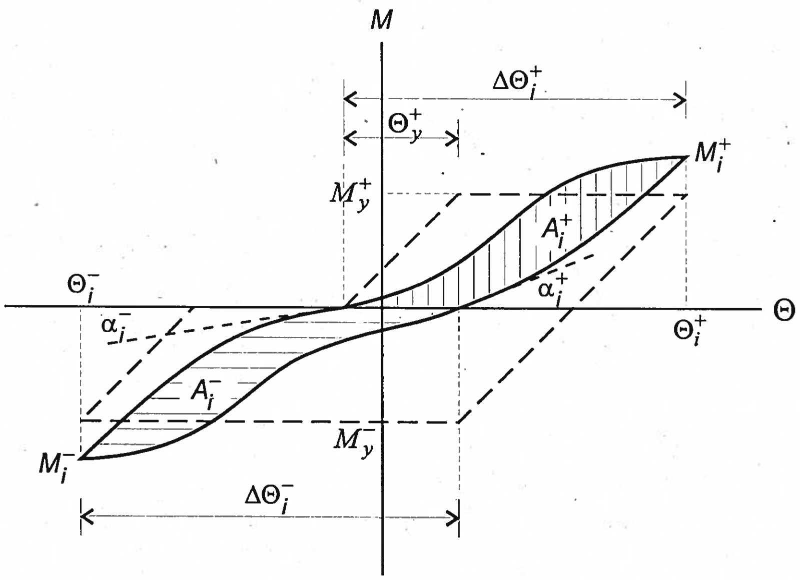

Figure 19 Notation for the calculation of cyclic characteristics

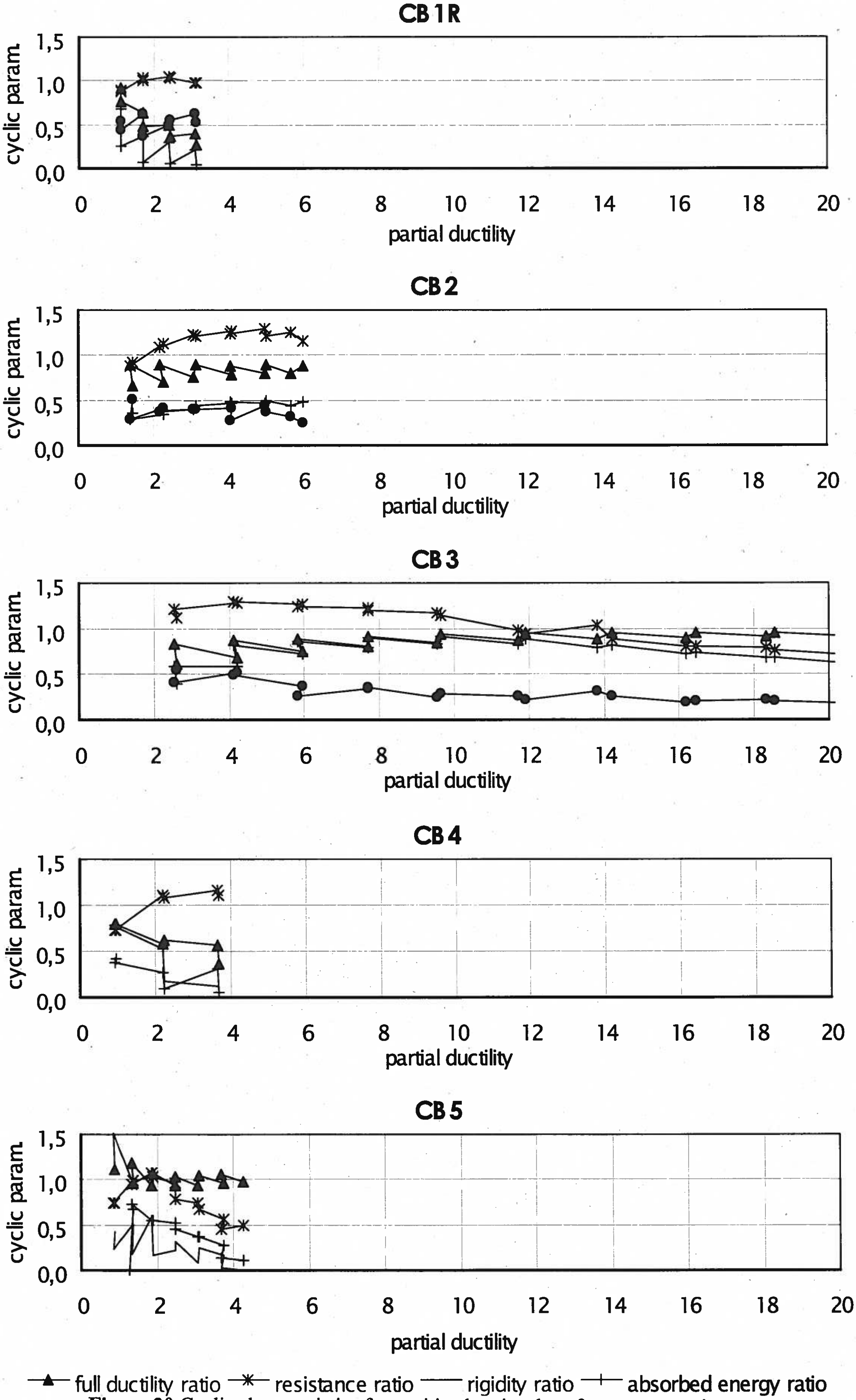

Figure 20 Cyclic characteristics for positive hemicycles of moment-rotation curves

Ductility

In case of bolt behaviour (CB4) the full ductility ratio is rapidly decreasing during the consecutive cycles. This degradation is in connection with the rigid-body-type rotation of the joint, as it is described later.

In the other cases the value of the full ductility ratio is near to 1, which corresponds to a ductile behaviour.

Resistance

Generally it can be stated that all the cases represent good cyclic behaviour from the viewpoint of resistance. The value of the resistance ratio is usually more than 1, without considerably degradation, which means that the moment resistance of the joint does not change significantly even after several plastic cycles of loading.

The only exception is the case of the end-plate behaviour (CB5). In this case the decreasing tendency is definitely caused by the flange weld crack, which resulted in a reduced end-plate cross-section at the welds, consequently, a reduced resistance of the end-plate and the joint.

Rigidity

In practically all the cases considerable rigidity degradation can be observed.

In case of governing bolt behaviour (CB4, CB1R) a rigid-body-type rotation clearly can be observed in the moment-rotation diagram. Practically it means a horizontal part of the diagram at the zero force level.

When the bolt behaviour is combined with the end-plate behaviour the rigid-body-type rotation does not occur. However, the rigidity is strongly reduced.

From rigidity point of view CB3 shows the best behaviour. There is no rigid-body-type rotation at all, although the rigidity is continuously decreasing due to the deterioration of the whole column section.

Absorbed energy

In case of governing bolt behaviour (CB1R, CB4) the absorbed energy ratio rapidly decreases during the consecutive cycles. Another important observation that in the repeated cycles (with the same maximal displacement) there is a significant drop of the ratio, which indicates that there is almost no energy dissipation in these repeated cycles. The phenomenon can be drawn back to the rigid-body-type displacement, as discussed above.

In case of CB2 (combined end-plate and bolt behaviour) the value of the absorbed energy ratio is approx. equal to 0.5, constantly. The important thing is that there is no degradation, in this case.

The diagram for CB5 shows a decreasing tendency. This degradation is certainly caused by the weld cracks, which reduced the area of the end-plate, resulted in reduced moment resistance and energy dissipation capacity.

The most advantageous behaviour belongs to CB3, when the local buckling of column flanges/web determine the behaviour. Even after several plastic cycles the absorbed energy ratio is more than 0.5, although it has a decreasing tendency if the partial ductility is greater than 10-12.

5 CONCLUSION

In this paper an experimental program on steel bolted end-plate joints is presented with the primary aim of providing information on the behavioural components which determine the joint behaviour. Here, some general conclusions are drawn.

It can be stated that the experienced behaviour of each specimen is in accordance with the expected behaviour. Thus, the applied method for the preliminary calculation is justified. The five different specimens cover a wide range of behaviour, including governing bolt behaviour (CB1 and CB4), governing base-plate behaviour (CB5), column local buckling (CB3) and a combined base-plate/bolt behaviour (CB2).

The tests justified the existence of the three basic types of behaviour. (Note that the concrete behaviour was not investigated in the present study.) However, the important effect of weld cracks is also highlighted. Whenever there is intensive end-plate deformation the failure is caused by the cracks occurred at the flange to end-plate welds. The cracks also influence the cyclic characteristics causing significant degradation of the moment resistance and the energy absorption capacity. Thus, although the end-plate behaviour would have good cyclic characteristics (since it is determined by the steel material behaviour), the weld cracks can strongly modify the behaviour.

The most advantageous behaviour is experienced if the deformations are concentrated in the column section, forming a plastic hinge (CB3, governing column behaviour). In this case the behaviour is extremely ductile, with considerable energy absorption capacity. On the other hand, whenever there is significant bolt elongation, the rigidity and energy dissipation capacity of the joint considerably decrease, due to the rigid-body-type rotation of the joint. In this case also the deformation capacity is reduced, as a consequence of the limited elongation capacity of the bolts.

The obtained results are applicable for the verification and calibration of numerical models. Detailed experimental data are provided for various behaviour types corresponding to the same joint topology. It is important, however, to study the concrete behaviour, which can be the topic of further investigations.

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